

# TOWN OF OHATCHEE

P.O. Box 645

Ohatchee, Alabama 36271

256-892-3233

*"Biggest Little Town in the U.S.A."*

Steven E. Baswell

Mayor

Celesia S. Jennings

*Council Member Place No. 1*

J.M. "Butch" Mitchell

*Council Member Place No. 2*

Anthony Crosson

*Council Member Place No. 3*

Teresa G. Lott

*Council Member Place No. 4*

Perry R. Howard

*Council Member Place No. 5*

August 14, 2023

## PUBLIC MEETING NOTICE

The Town of Ohatchee will be hosting a Public Meeting on Tuesday, September 12, 2023, at 5:00 p.m. The meeting will be held at the Ohatchee Town Hall located at 7801 Highway 77, Ohatchee, Alabama 36271. The Public Meeting will be held to receive comments on a Tower Replacement to be installed at 7622 Alabama Highway 77, Ohatchee, AL 36271, proposed by PowerSouth- COOP Member Coosa Valley.

Sincerely,



Mayor Steve Baswell

256-892-3233

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Council Member Place No. 5

## SPECIAL CALLED HEARING

SEPTEMBER 12, 2023

### SIGN IN SHEET

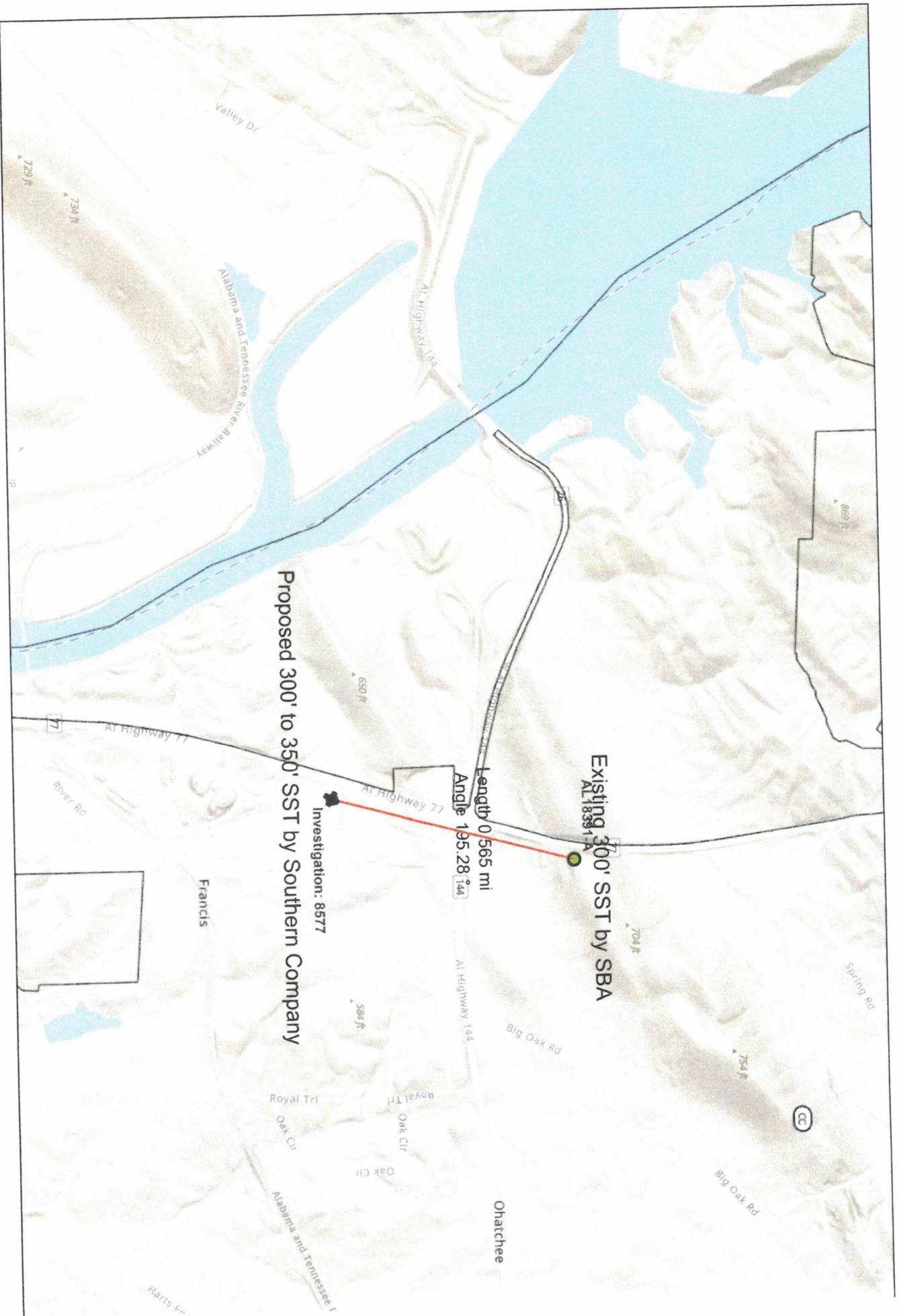
NAME

PHONE

EMAIL

|                   |                |                               |
|-------------------|----------------|-------------------------------|
| Juan Faber        | 773 704 2132   | Fabers@dr.com                 |
| James A. Harris   | (205) 901-6242 | jharris@votk.wregea.com       |
| Anthony Crosson   | 256-239-1003   | acrosson195@gmail.com         |
| Perry Howard      | 256-282-3277   | Perry.howard2412@gmail.com    |
| Butch Mitchell    | 256-892-3354   |                               |
| Teresa Lott       | 256-454-1738   |                               |
| Celesia Jennings  | 256-282-7532   | CelesiaJennings@gmail.com     |
| Chief Kelly       | 256-689-0349   | ChiefKelly@townofohatchee.com |
| James Harris      | 205-505-9303   |                               |
| Sybil Harris      | 205-505-9413   |                               |
| William T. Faber  | 630 440478     |                               |
| Helen Lott        | 256-892-4424   |                               |
| Raymond Underwood | 832-738-6950   | underwoodhome@verizon.net     |
| St Baswell        |                |                               |

# Town of Ohatchee - INV 877



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Distance & Direction - Line Graphics

Overrid

Geographic Jurisdiction

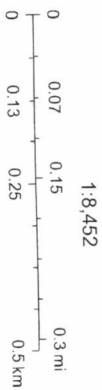
U.S. Competitors



# Town of Ohatchee - INV 8577



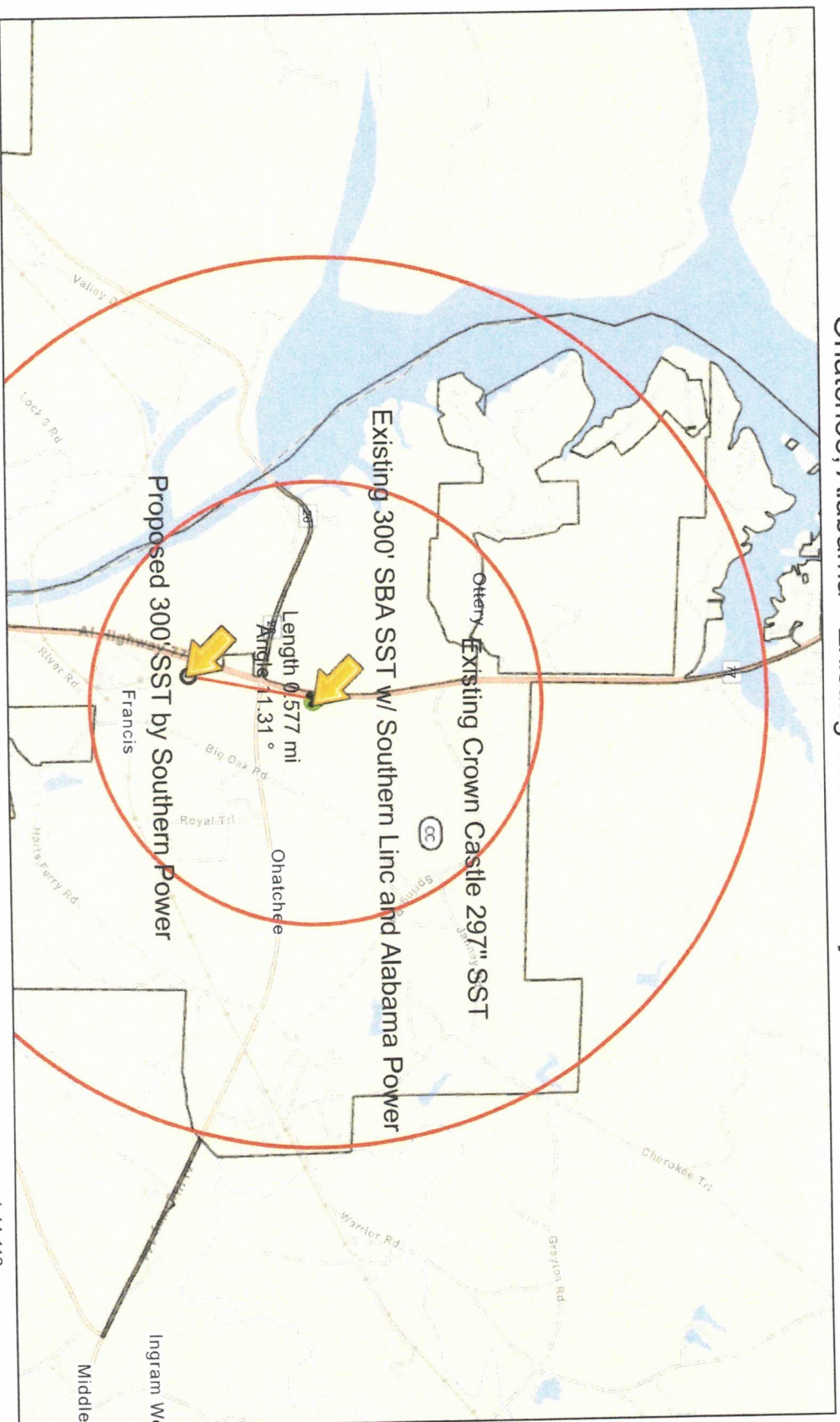
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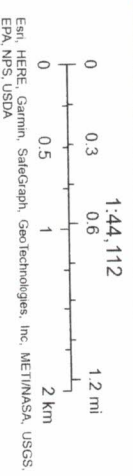
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# Ohatchee, Alabama - Existing Towers within jurisdiction



9/8/2023, 12:01:57 PM



TO: SBA

Subject: RF Coverage Plot Analysis: City of Ohatchee, AL

Date: September 11, 2023

1. **METHODOLOGY.** The attached plots depict broadcast radio frequency (RF) coverage from the existing site to City of Ohatchee, AL and a proposed site approximately 0.57 miles to the South. At each location, cellular industry typical LTE operating parameters were considered for omnidirectional antennas mounted at 290 feet above ground level at the existing site, and at 290 feet above ground level for the proposed site. Ground elevations are 608 and 511 feet above mean sea level, respectively.

The provided predictive coverage plots were created using established radiowave propagation models with terrain and clutter data, implemented by qualified engineers specializing in this discipline and having years of US Industry experience.

Plots for both locations were generated for 700, 850, 1900 and 2100 MHz operations. The signal levels depicted are associated with LTE service reliability where the strong coverage levels in green and blue occur near the towers and decrease with distance from the sites and intervening terrain obstructions. Signal levels greater than -70 dBm shown as blue are associated with feasible coverage within buildings. Marginal coverage is provided in the regions depicted in yellow between -90 dBm and -80 dBm and signal levels between -100 dBm and -90 dBm shown as red represent poor coverage associated with call failures.

A comparison of coverage performance for each site is based on low band (700 and 850 MHz) and high band (1900 and 2100 MHz) prediction results. Radiowave propagation conditions between these bands differ because of terrain and ground clutter (e.g. vegetation) effects at different frequencies. Generally, low band operations provide greater area coverage. Therefore, high band operations provide additional customer traffic capacity closer to the cellular site.

2. **COMPARISON.** The sites considered in these coverage plots provide service to the depicted locations and roads leading to City of Ohatchee, AL. For low band operations, the existing site provides strong coverage approximately 0.3 miles in all directions from the tower and to non-contiguous areas to 0.4 miles from the site. This includes 1.3 miles of Alabama State Route 77. The proposed site provides strong coverage approximately 0.3 miles in all directions from the tower and to non-contiguous areas to 0.4 miles from the site. This includes 1.3 miles of Alabama State Route 77.

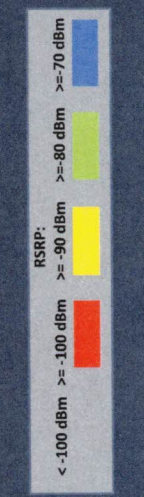
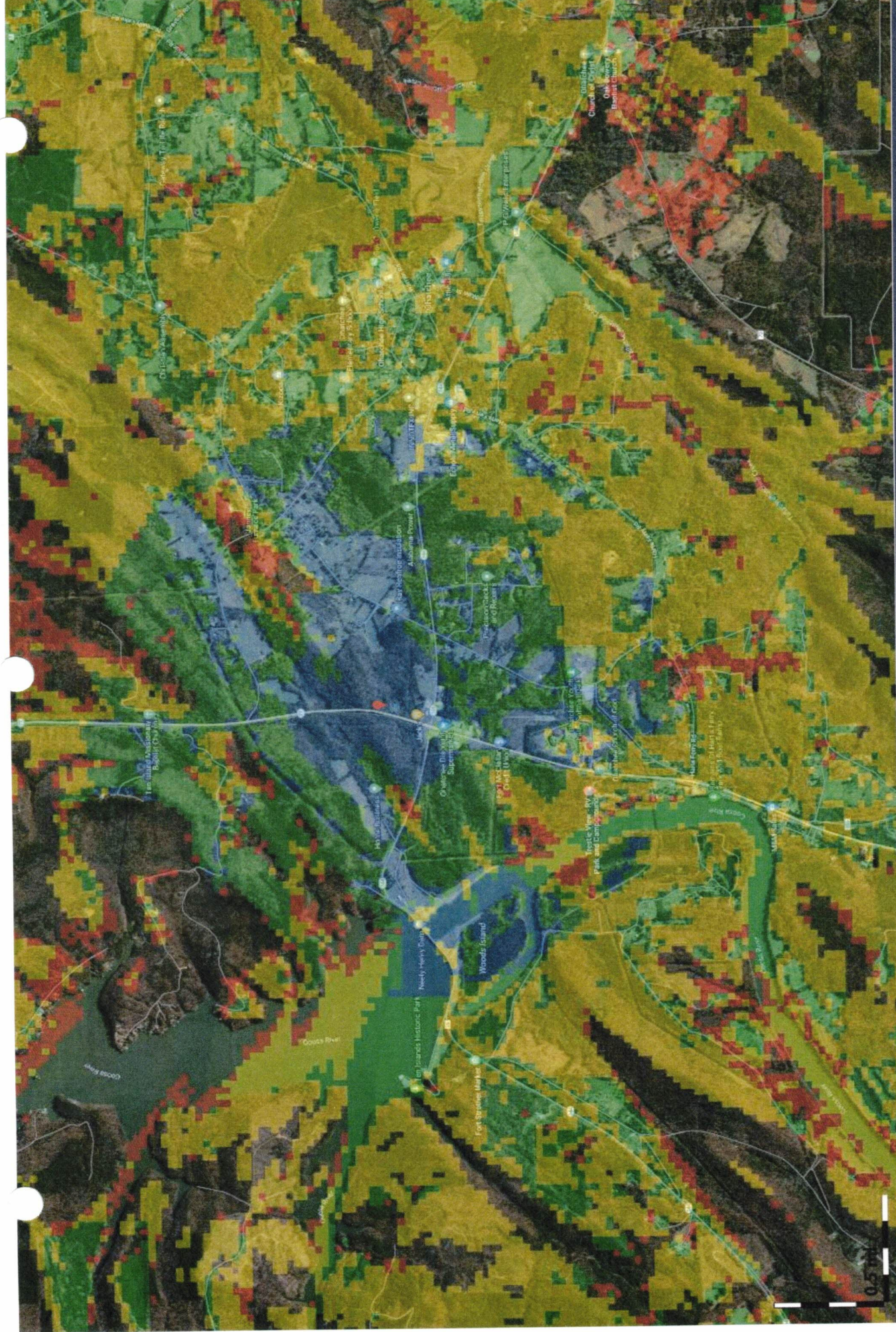
For high band operations, both sites provide strong coverage to non-contiguous areas 0.4 miles in all directions from the tower.

3. **CONCLUSION.** Based on the coverage comparison presented above, the proposed site provides comparable coverage to City of Ohatchee, AL due to its close proximity to the existing site. For wireless



operators with antennas mounted on the existing site, the installation of additional antennas on the proposed site would be considered to be an "overbuild" or impractical given the coverage overlap.



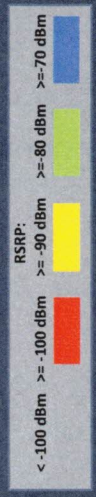
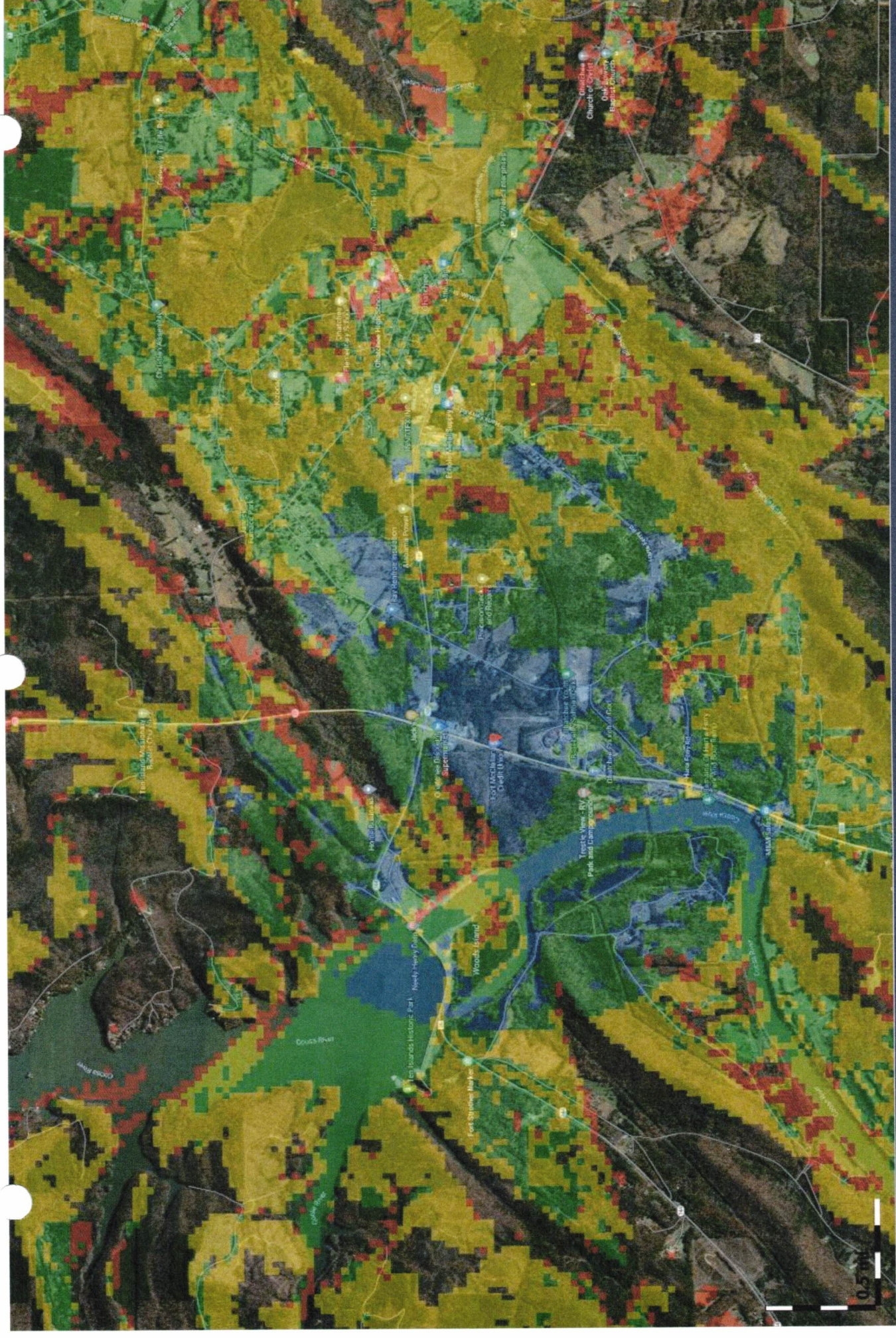


Site Name: AL18391-A Ohatchee  
 Latitude: N33.786846  
 Longitude: W86.035103

Antenna: 15.15 dBi Omni  
 Alpha Rad Center (ft): 290  
 Azimuth (Deg): 0  
 ERP per RS (W): 2.0

Existing Site  
 700 MHz Coverage





Antenna: 15.15 dBi Omni  
Alpha Rad Center (ft): 290  
Azimuth (Deg): 0  
ERP per RS (W): 2.0

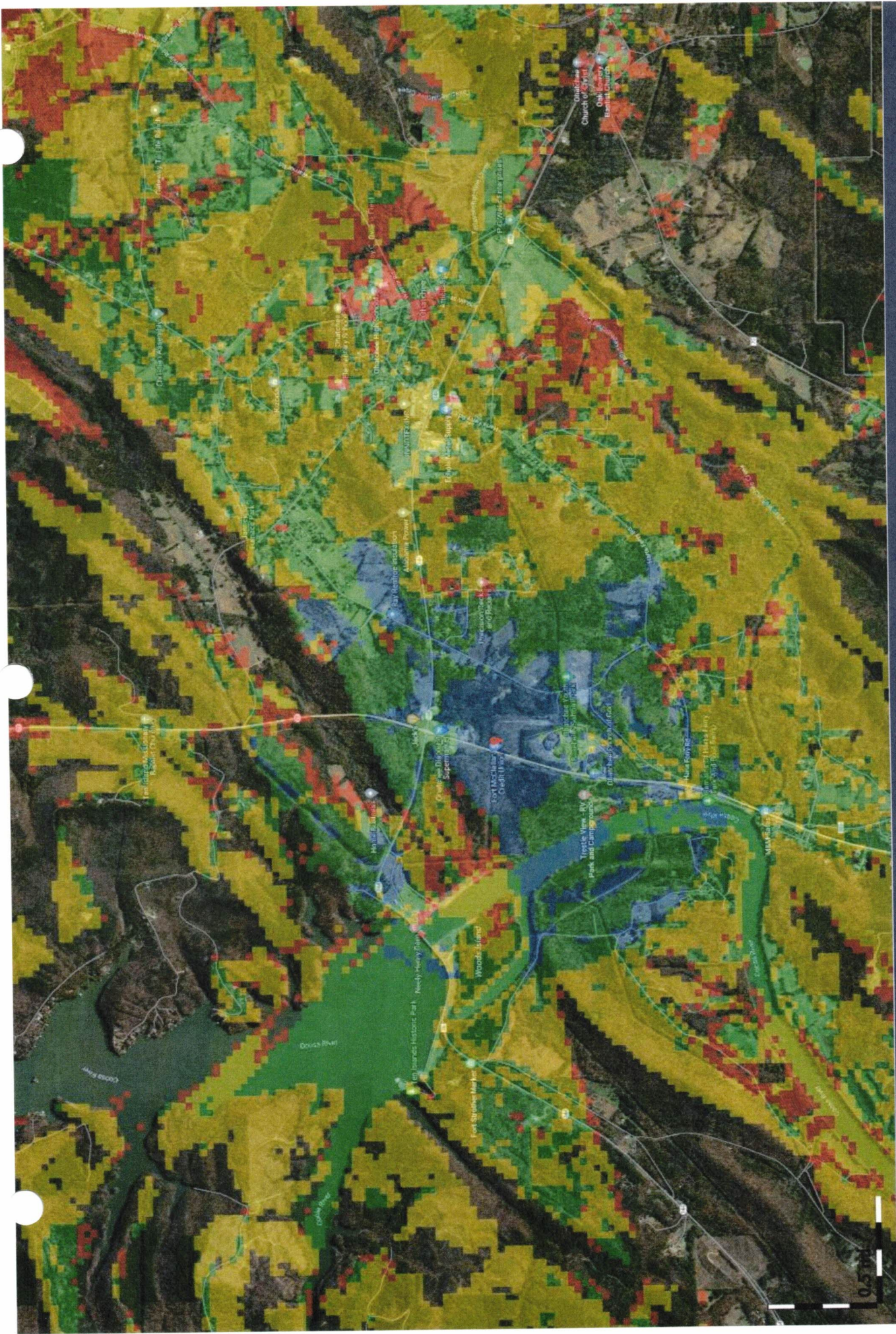
Proposed  
Latitude: N33.778944  
Longitude: W86.037667

Proposed Site  
700 MHz Coverage









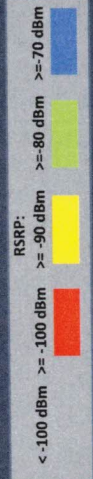
# Proposed Site 850 MHz Coverage

Site Name  
Latitude:  
Longitude:

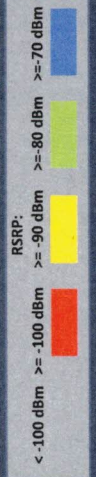
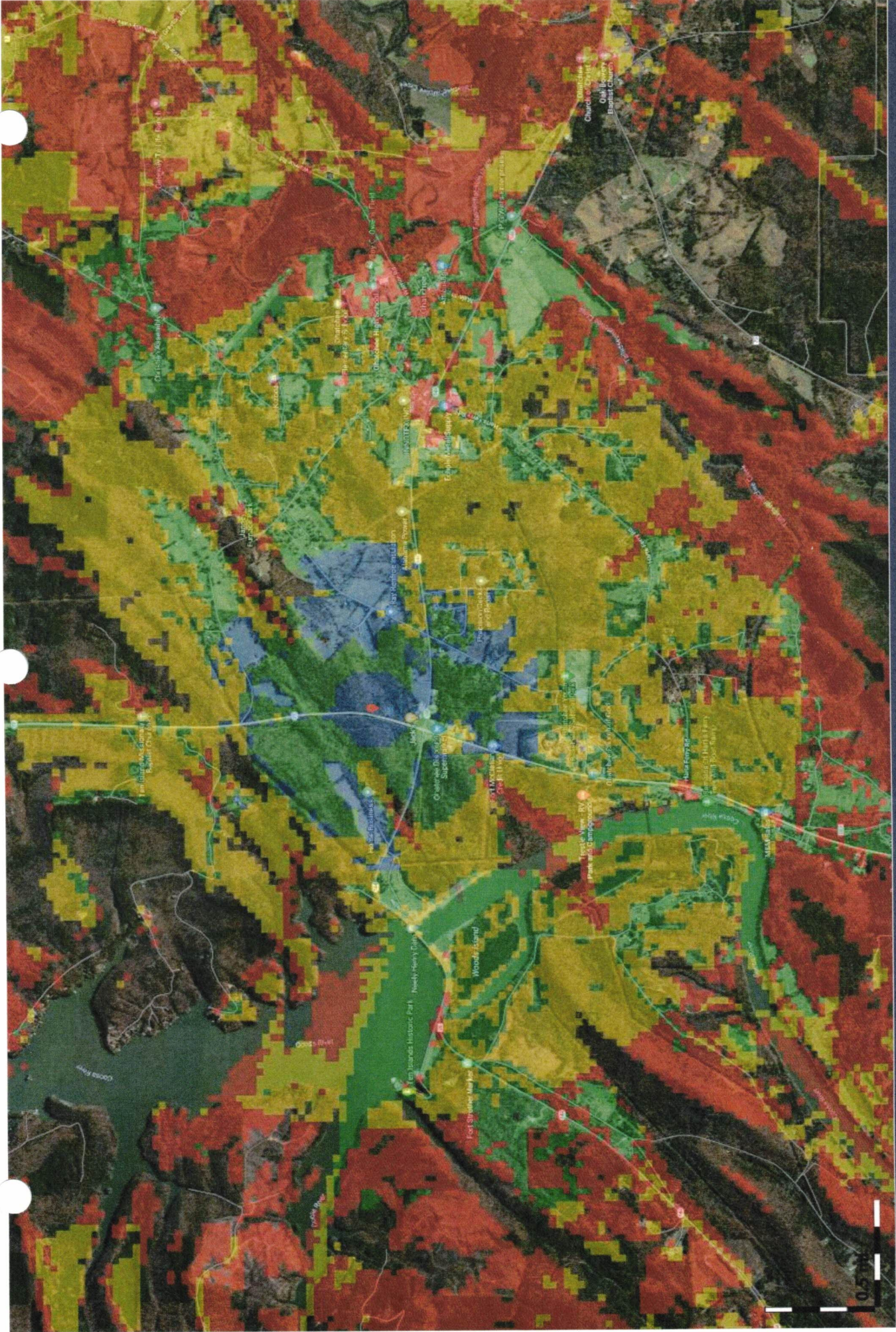
Proposed  
N33.778944  
W86.037667

Antenna:  
Alpha Rad Center (ft):  
Azimuth (Deg):  
ERP per RS (W):

15.15 dBi Omni  
290  
0  
2.0





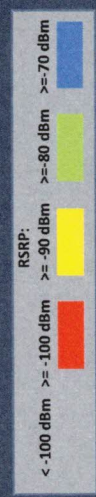
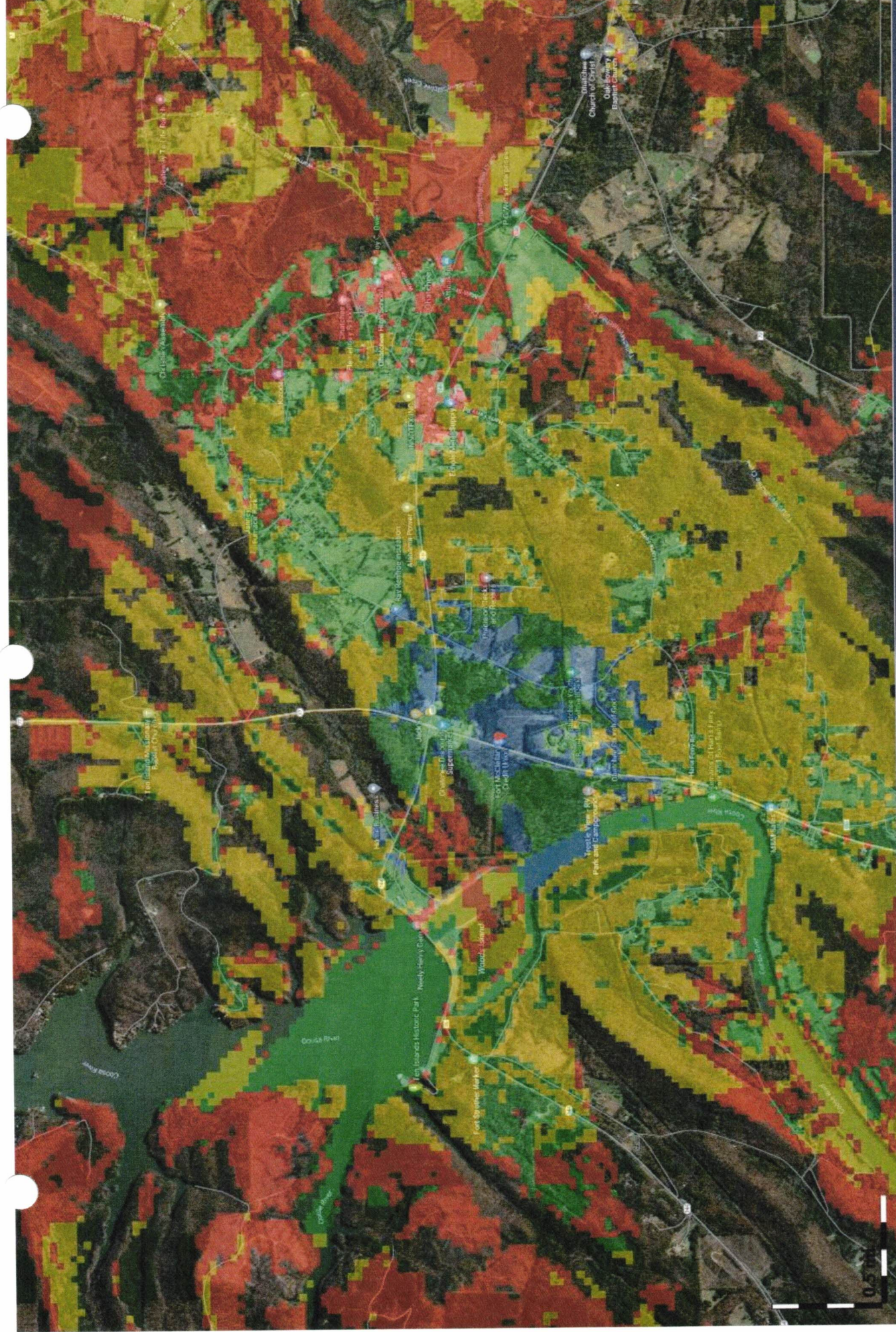


Antenna: 17.15 dBi Omni  
Alpha Rad Center (ft): 290  
Azimuth (Deg): 0  
ERP per RS (W): 4.7

Site Name AL18391-A Ochatsee  
Latitude: N33.786846  
Longitude: W86.035103

Existing Site  
1900 MHz Coverage



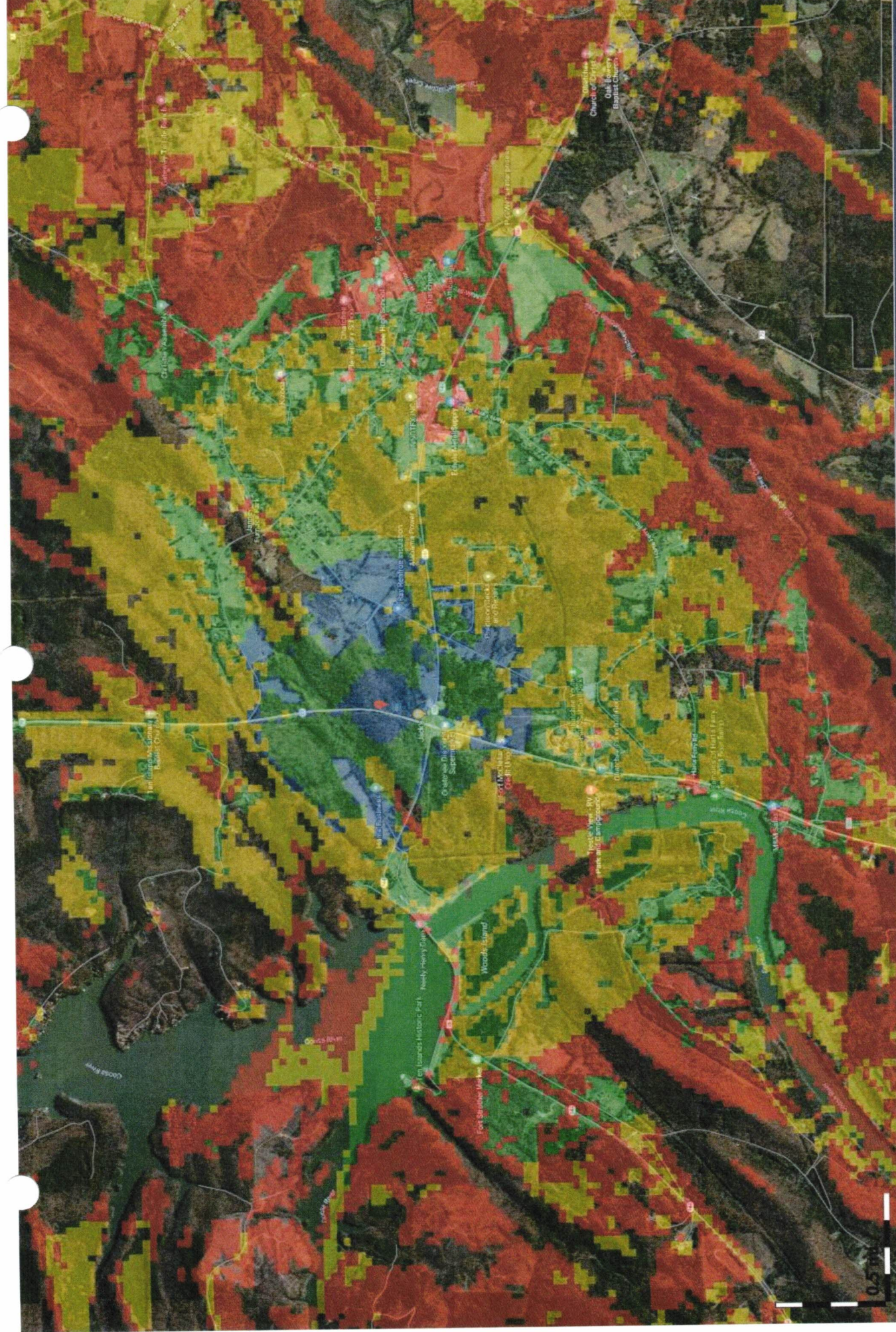


Antenna: 17.15 dBi Omni  
Alpha Rad Center (ft): 290  
Azimuth (Deg): 0  
ERP per RS (W): 4.7

Proposed  
Latitude: N33.778944  
Longitude: W86.037667

Proposed Site  
1900 MHz Coverage



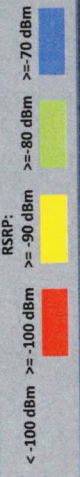
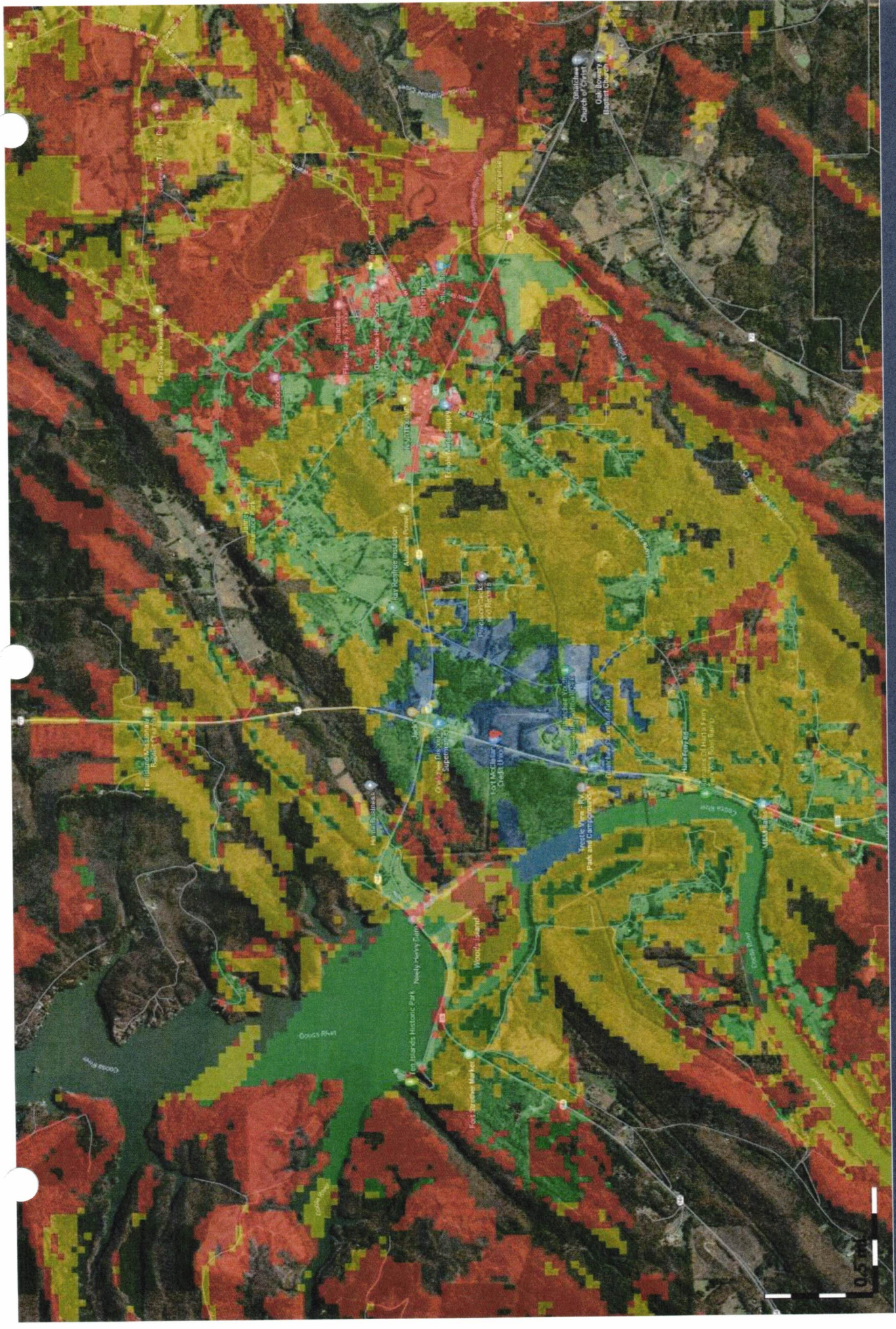


Antenna: 17.15 dBi Omni  
Alpha Rad Center (ft): 290  
Azimuth (Deg): 0  
ERP per RS (W): 4.7

Site Name AL18391-A Ohatchee  
Latitude: N33.786846  
Longitude: W86.035103

Existing Site  
2100 MHz Coverage





Antenna: 17.15 dBi Omni  
 Alpha Rad Center (ft): 290  
 Azimuth (Deg): 0  
 ERP per RS (W): 4.7

Proposed  
 Site Name: N33.778944  
 Latitude: W86.037667  
 Longitude:

Proposed Site  
 2100 MHz Coverage





SBA Communications Corporation  
8051 Congress Avenue  
Boca Raton, FL 33487-1307

T + 561.995.7670  
F + 561.995.7626

## Structural Analysis Report

sbasite.com

### Client: Alabama Power

Client Site ID: AL18391-A  
Client Site Name: Ohatchee (AL Power)  
ApplD: 9248, v1

SBA Site Name: Ohatchee  
SBA Site ID: AL18391-A  
300' Self Supporting Tower  
8130 Alabama Highway 77  
Ohatchee, AL 36271  
Lat: 33°47'12.65", Long: -86°2'6.37"

Project number: AL18391-AL-073118

### Analysis Results

|            |        |      |
|------------|--------|------|
| Tower      | 55.90% | Pass |
| Foundation | 37.60% | Pass |

Client Mount modification / replacement

|  |     |
|--|-----|
| Net change in tower stress due to mount Modification / replacement | N/A |
|--|-----|

### Twist and Sway with a 10 dB Degradation Limit

| Elev. (ft) | Model                    | Frequency (GHz) | Calculated Twist/Sway (°) | Allowable Twist/Sway (°) | Analysis Results |
|------------|--------------------------|-----------------|---------------------------|--------------------------|------------------|
| 280        | ANT2 0.9 HPX 10/1 - Dish | 10.700          | 1.654                     | 0.376                    | Sufficient       |
|            |                          | 11.700          | 1.513                     | 0.376                    | Sufficient       |
| 280        | ANT2 0.6 HPX 18 - Dish   | 21.200          | 1.246                     | 0.376                    | Sufficient       |
|            |                          | 23.600          | 1.125                     | 0.376                    | Sufficient       |

\* Client must review the operational limits of the Microwave dish

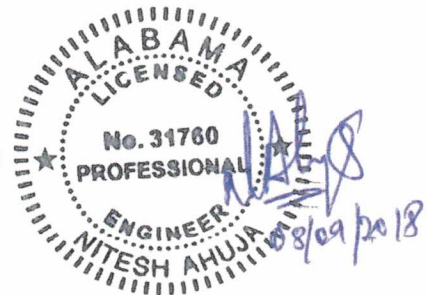
\*\*Frequencies are assumed

Prepared by:

Serge Berthomieux  
Structural Analyst  
561-226-9365  
SBerthomieux@sbasite.com

Reviewed by:

Nitesh Ahuja, PE  
Director of Engineering  
561-226-9452  
nahuja@sbasite.com



August 9, 2018

Prepared in compliance with:

- ANSI/TIA-222-G Structural Standard for Antennas and Antenna Supporting Structures
- 2009 International Building Code (IBC)



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## Executive Summary

The enclosed structural analysis was performed for *Alabama Power* on *August 9, 2018* to verify the structural capacity of the 300' Self Supporting Tower located at 8130 Alabama Highway 77, Ohatchee, AL 36271 to support the proposed antenna, transmission lines and mounting equipment in addition to those currently installed. The following documents were used to determine the geotechnical characteristics, foundation data, tower geometry and member sizes/type:

Table 1 List of Documents Used

| Item                         | Document   |
|------------------------------|--|
| <b>Tower design/drawings</b> | World Tower, Job #: Q15-366, Dated: 5/19/2015                          |
| <b>Foundation drawings</b>   | World Tower, Job #: Q15366N, Dated: 5/21/2015                          |
| <b>Geotechnical report</b>   | Environmental Corporation of America, Project # R0441, Dated: 5/7/2015 |
| <b>Latest SA</b>             | N/A  |

The analysis was performed in accordance with the following requirements:

Table 2 Code Related Data

|   |                              |
|---|------------------------------|
| <b>Jurisdiction (State/County/City)</b> | Alabama / Calhoun / Ohatchee |
| <b>Governing Codes</b>                  | ANSI/TIA-222-G , 2009 IBC    |
| <b>Base Wind Speed</b>                  | 90 mph                       |
| <b>Wind Speed with Ice</b>              | 30 mph                       |
| <b>Ice Thickness</b>                    | 0.5 in                       |
| <b>Structural Class</b>                 | II                           |
| <b>Exposure Category</b>                | C                            |
| <b>Topographic Category</b>             | 1                            |
| <b>Crest Height</b>                     | 0 ft                         |

"This structural analysis is based upon the tower being classified as a class II; however, if a different classification is required subsequent to the date hereof, the tower classification will be changed to meet such requirement and a new structural analysis will be run."

The SBA Communications Corporation verifies that the 300' Self Supporting Tower located at 8130 Alabama Highway 77, Ohatchee, AL 36271 is **sufficient** to support the proposed loadings for Alabama Power in addition to those currently existing based on standards set forth in governing building codes and dependent on Alabama Power satisfying all Installation Requirements provided herein. The analysis performed assumes the site information provided is accurate and the tower/foundation has been properly designed, manufactured, installed and maintained. Additional details regarding the assumptions and limitations are provided within the Assumptions and Limitations section of this report.

## Assumptions

This analysis was completed based on the following assumptions:

- Tower has been properly maintained
- Tower erection was in accordance to manufacturer drawings
- Leg flanges have been properly designed by manufacturer to not be a limiting reaction
- Welds have been properly designed and installed by manufacturer to not be a limiting reaction
- Foundation was constructed in accordance to manufacturer drawings
- Foundation does not have structural damage
- Bolts have been properly tightened according to manufacturer specifications
- Appurtenance, mount and transmission line sizes and weights are best estimates using the tnxTower database and manufacturer information



## Limitations

The computer generated analysis performed by the tnxTower software is limited to theoretical capacities of the towers structural members and does not account for any missing or damaged members or connections. The tower and foundation are assumed to have been properly designed, fabricated, installed and maintained, barring any conflicting findings from the most recent inspection. All leg flanges, welds and bolts are assumed to be designed by the manufacturer in such a way that these are not limiting reactions.

SBA Communications Corporation has used its due diligence to verify the information provided to perform this analysis. It is unreasonable to perform a more detailed inspection of a tower and its components. This report is not a condition assessment of the tower or foundation.

## Installation Requirements

This analysis was performed under the assumption that Alabama Power will place the proposed equipment and feed lines at a height of 280' and in accordance with the coax layout shown. RRUs are to be installed on existing mounts behind tenant's antennas unless otherwise noted. No equipment is to be installed directly in the climbing path. All equipment is to be installed per mount manufacturer specifications. In case site conditions do not allow for the required installation parameters to be met Alabama Power must notify SBA Communications Corporation engineers for approval of an alternative placement.



## Appurtenance Loading

### Existing Loading:

The existing antenna and feed line information was obtained from the Site Summary and/or previous Structural Analysis. SBA Communications Corporation uses due diligence to ensure reasonably accurate information has been recorded. The existing loadings are shown in Table 3.

Table 3 Existing Appurtenances

| Mount Elev. (ft) | CL Elev. (ft) | Carrier       | Type  | Qty      | Manufacturer               | Model        | Qty | Feed Line Size | Mount Type Qty |
|------------------|---------------|---------------|-------|----------|----------------------------|--------------|-----|----------------|----------------|
| 294              | 294           | Southern Linc | Panel | 6        | Kathrein                   | 800 10735V01 | 1   | 7/8" Hybrid    | 2.5' Standoff  |
|                  | RRU           |               | 3     | Ericsson | RRUS 11                    |              |     |                |                |
|                  | Filter        |               | 3     | Ericsson | Public Safety Notch Filter |              |     |                |                |
| 280              | 280           |               | Dish  | 2        | Antel                      | ANT1 0.9 11  | 2   | 3/8"           | (1) Pipe Mount |
|                  |               |               | ODU   | 1        | Ericsson                   | 20150720 ODU |     |                |                |

\*(3) Kathrein 800 10735V01 & (1) Antel ANT1 0.9 11 are reserved loading

### Proposed Loading:

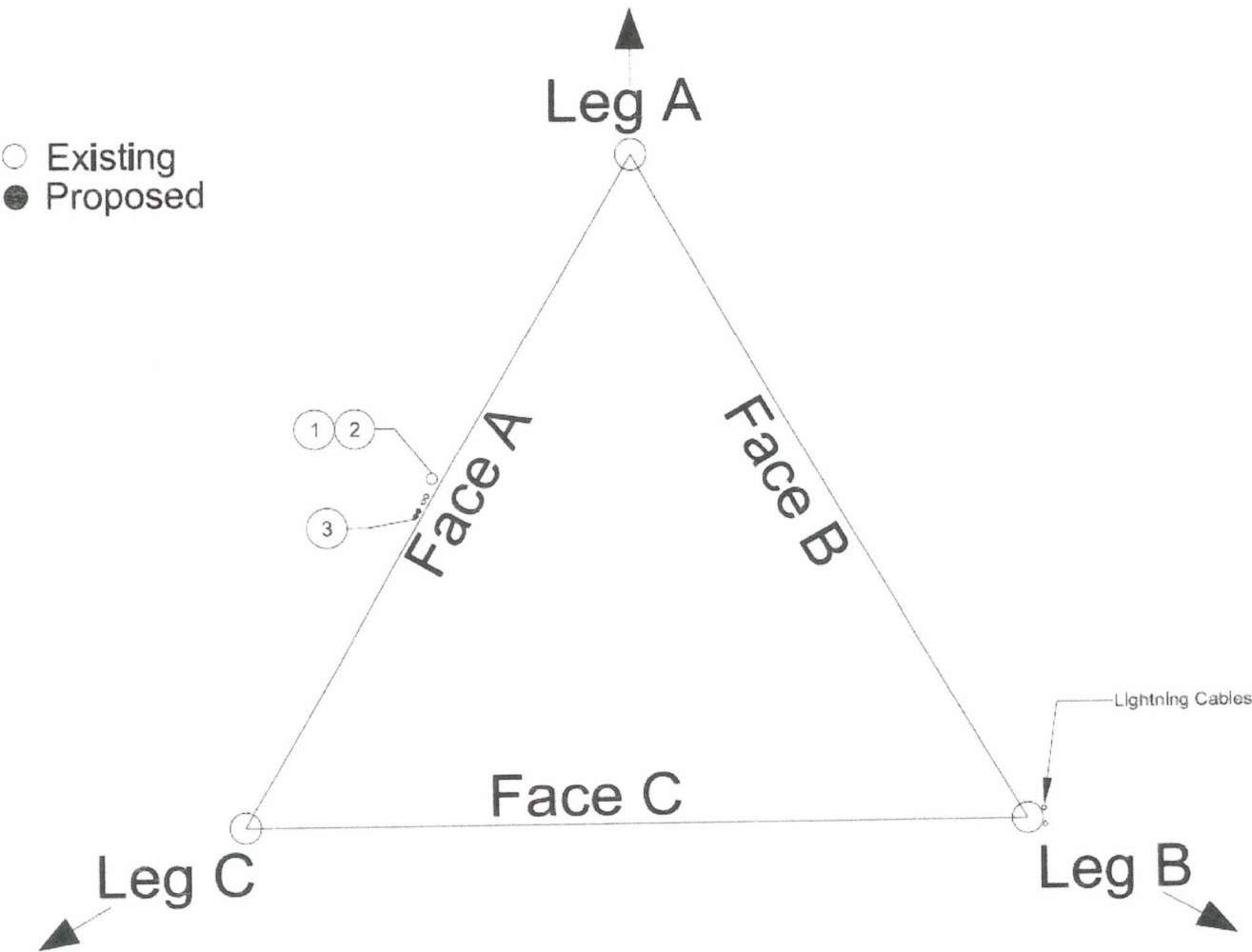
Information pertaining to proposed antennas and transmission lines were based upon the APP ID 92481 v1 from Alabama Power and is listed in Table 4.

Table 4 Proposed Appurtenances

| Mount Elev. (ft) | CL Elev. (ft) | Carrier       | Type | Qty | Manufacturer | Model           | Qty | Feed Line Size | Mount Type Qty |
|------------------|---------------|---------------|------|-----|--------------|-----------------|-----|----------------|----------------|
| 280              | 280           | Alabama Power | Dish | 1   | Ericsson     | ANT2 0.6 HPX 18 | 2   | 3/8"           | (1) Pipe Mount |



Coax Layout



| AL18391-A |               |      |      |           |          |
|-----------|---------------|------|------|-----------|----------|
| #         | CARRIER       | SIZE | QTY. | ELEVATION | NOTES    |
| 1         | Southern Linc | 7/8" | 1    | 294'      | Hybrid   |
| 2         | Southern Linc | 3/8" | 2    | 280'      |          |
| 3         | Alabama Power | 3/8" | 2    | 280'      | Proposed |



## Results

### Tower

The results of the structural analysis performed with the tnxTower software are shown below. Table 5 shows the most critical member elements and the percentage of the force in the member with respect to the member capacity. Capacities of up to 105% are considered acceptable. The foundation reactions obtained from tnxTower are shown in Table 6 and Table 7. Table 8 displays the twist and sway at service wind speeds. These reactions are used for the analysis of the foundation systems. Additional information for the tower analysis is provided within the Appendix.

*Table 5 Tower Analysis Summary*

| Structural Component | % capacity | Analysis Result |
|----------------------|------------|-----------------|
| Leg                  | 46.80      | Pass            |
| Diagonal             | 55.90      | Pass            |
| Horizontal           | 45.20      | Pass            |
| Top girt             | 12.50      | Pass            |
| Bottom girt          | 14.20      | Pass            |
| Anchor Bolt          | 37.40      | Pass            |
| Bolt                 | 49.70      | Pass            |
| Tower                | 55.90      | Pass            |

*Table 6 Tower Base Reactions*

|                 |      |
|-----------------|------|
| Axial (kips)    | 65   |
| Shear (kips)    | 45   |
| Moment (kip-ft) | 6355 |

*Table 7 Tower leg Reactions*

|                    |     |
|--------------------|-----|
| Uplift (kips)      | 248 |
| Compression (kips) | 304 |
| Shear (kips)       | 28  |

*Table 8 Twist and Sway with a 10 dB Degradation Limit (for dishes only)*

| Elev. (ft) | Model                    | Frequency (GHz) | Calculated Twist/Sway (°) | Allowable Twist/Sway (°) | Analysis Results |
|------------|--------------------------|-----------------|---------------------------|--------------------------|------------------|
| 280        | ANT2 0.9 HPX 10/1 - Dish | 10.700          | 1.654                     | 0.376                    | Sufficient       |
|            |                          | 11.700          | 1.513                     | 0.376                    | Sufficient       |
| 280        | ANT2 0.6 HPX 18 - Dish   | 21.200          | 1.246                     | 0.376                    | Sufficient       |
|            |                          | 23.600          | 1.125                     | 0.376                    | Sufficient       |

\* Client must review the operational limits of the Microwave dish

\*\*Frequencies are assumed

### Foundation System

The results of the foundation based on the geotechnical report and foundation mapping or design drawings are shown below in Table 9. Additional information for the foundation analysis is provided within the Appendix.

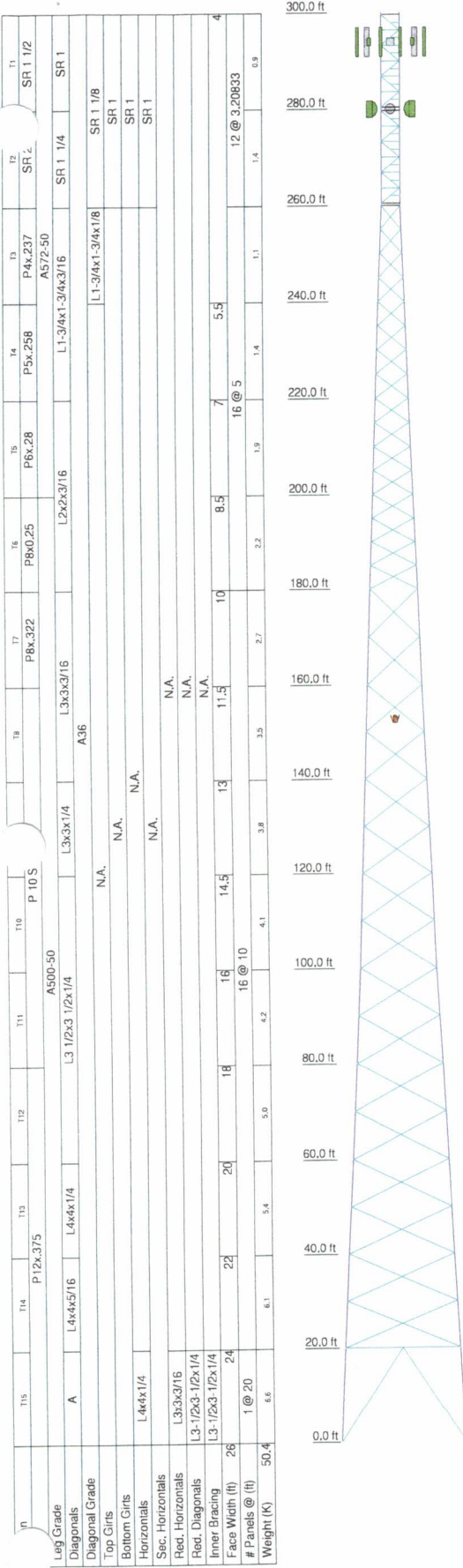
*Table 9 Foundation Analysis Summary*

| Structural Component | % capacity | Analysis Result |
|----------------------|------------|-----------------|
| Foundation           | 37.60      | Pass            |



## Appendix





DESIGNED APPURTENANCE LOADING

| TYPE   | ELEVATION | TYPE                                       | ELEVATION |
|--|-----------|--|-----------|
| Flash Beacon Lighting                          | 300       | 3' Standoff                                | 294       |
| Flash Beacon                                   | 300       | 2.4" X 7' Kickers                          | 294       |
| (2) 80010736V01 (96"x11.9"x3.9")               | 294       | 2.4" X 7' Kickers                          | 294       |
| (2) 80010736V01 (96"x11.9"x3.9")               | 294       | 2.4" X 7' Kickers                          | 294       |
| (2) 80010736V01 (96"x11.9"x3.9")               | 294       | Dish Mount                                 | 280       |
| RRUS 11 (17.8"x17"x7.3")                       | 294       | Dish Mount                                 | 280       |
| RRUS 11 (17.8"x17"x7.3")                       | 294       | Dish Mount                                 | 280       |
| RRUS 11 (17.8"x17"x7.3")                       | 294       | 20150720 ODU                               | 280       |
| Public Safety Notch Filter (15" x 7.9" x 3.9") | 294       | ANT2 0.9 HPX 10/11 (39.3" x 39.3" x 24.3") | 280       |
| Public Safety Notch Filter (15" x 7.9" x 3.9") | 294       | ANT2 0.9 HPX 10/11 (39.3" x 39.3" x 24.3") | 280       |
| Public Safety Notch Filter (15" x 7.9" x 3.9") | 294       | ANT2 0.6 HPX 18 (26.1" x 26.1" x 22.6")    | 280       |
| 3' Standoff                                    | 294       | Side Light                                 | 152       |
| 3' Standoff                                    | 294       | Side Light                                 | 152       |

SYMBOL LIST

| MARK | SIZE                  | MARK | SIZE |
|------|-----------------------|------|------|
| A    | 2L3-1/2x3-1/2x1/4x1/2 |      |      |

MATERIAL STRENGTH

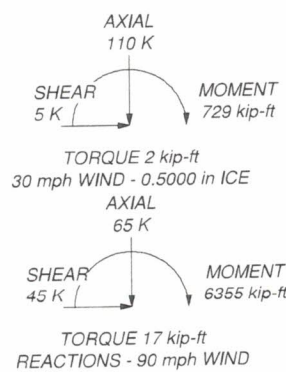
| GRADE   | Fy     | Fu     | GRADE   | Fy     | Fu     |
|---------|--------|--------|---------|--------|--------|
| A572-50 | 50 ksi | 65 ksi | A500-50 | 50 ksi | 62 ksi |
| A36     | 36 ksi | 58 ksi |         |        |        |

TOWER DESIGN NOTES

1. Tower is located in Calhoun County, Alabama.
2. Tower designed for Exposure C to the TIA-222-G Standard.
3. Tower designed for a 90 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 30 mph basic wind with 0.50 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 55.9%

ALL REACTIONS  
ARE FACTORED

MAX. CORNER REACTIONS AT BASE:  
DOWN: 304 K  
SHEAR: 28 K  
  
UPLIFT: -248 K  
SHEAR: 23 K



**SBA Communications**  
8051 Congress Avenue  
Boca Raton, FL 33487  
Phone: (561) 226-9365  
FAX:

Job: **AL18391-A, Ohatchee**

Project: **AL18391-AP-073118**

Client: Alabama Power ( Application #: 92481, v1)

Code: TIA-222-G

Path:

Drawn by: sberthomieux

Date: 08/09/18

App'd:

Scale: NTS

Dwg No. E-1



|   |         |   |                             |
|---|---------|---|-----------------------------|
| <b>tnxTower</b><br><br><b>SBA Communications</b><br>8051 Congress Avenue<br>Boca Raton, FL 33487<br>Phone: (561) 226-9365<br>FAX: | Job     | AL18391-A, Ohatchee                       | Page<br>1 of 27             |
|   | Project | AL18391-AP-073118                         | Date<br>16:38:08 08/09/18   |
|   | Client  | Alabama Power ( Application #: 92481, v1) | Designed by<br>sberthomieux |

## Tower Input Data

The main tower is a 3x free standing tower with an overall height of 300.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 4.00 ft at the top and 26.00 ft at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Calhoun County, Alabama.

Basic wind speed of 90 mph.

Structure Class II.

Exposure Category C.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 0.5000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 30 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

- |  |  |  |
|--|--|--|
| <ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>√ Include Bolts In Member Capacity</li> <li>Leg Bolts Are At Top Of Section</li> <li>√ Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>SR Members Have Cut Ends</li> <li>SR Members Are Concentric</li> </ul> | <ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>√ Use Clear Spans For KL/r</li> <li>Retension Guys To Initial Tension</li> <li>√ Bypass Mast Stability Checks</li> <li>√ Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>Autocalc Torque Arm Areas</li> <li>Add IBC .6D+W Combination</li> <li>√ Sort Capacity Reports By Component</li> <li>Triangulate Diamond Inner Bracing</li> <li>Treat Feed Line Bundles As Cylinder</li> </ul> | <ul style="list-style-type: none"> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>√ Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feed Line Torque</li> <li>√ Include Angle Block Shear Check</li> <li>Use TIA-222-G Bracing Resist. Exemption</li> <li>Use TIA-222-G Tension Splice Exemption</li> <li style="text-align: center;"><b>Poles</b></li> <li>Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> <li>Pole Without Linear Attachments</li> <li>Pole With Shroud Or No Appurtenances</li> <li>Outside and Inside Corner Radii Are Known</li> </ul> |
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